

1908
Smith

SMITH

Tests of Bond Between
Concrete and Steel

Civil Engineering


B. S.

1908

THE UNIVERSITY
OF ILLINOIS
LIBRARY

1908
Sm5





Digitized by the Internet Archive
in 2013

<http://archive.org/details/testsofbondbetwe00smit>

TESTS OF BOND BETWEEN
CONCRETE AND STEEL

BY

CLAIRE HOWLAND WALLACE SMITH

THESIS

FOR THE

DEGREE OF BACHELOR OF SCIENCE

IN

CIVIL ENGINEERING

COLLEGE OF ENGINEERING

UNIVERSITY OF ILLINOIS

PRESENTED, JUNE, 1908

1908
Sm5

UNIVERSITY OF ILLINOIS

June 1, 1908

THIS IS TO CERTIFY THAT THE THESIS PREPARED UNDER MY SUPERVISION BY

CLAIRE HOWLAND WALLACE SMITH

ENTITLED TESTS OF BOND BETWEEN CONCRETE AND STEEL

IS APPROVED BY ME AS FULFILLING THIS PART OF THE REQUIREMENTS FOR THE

DEGREE OF Bachelor of Science in Civil Engineering

A. M. Talbot

D. A. Abrams
Instructor in Charge

APPROVED:

Ira O. Baker

HEAD OF DEPARTMENT OF Civil Engineering

254309

TABLE OF CONTENTS.

I. INTRODUCTION.

- | | | |
|----------------------|------|----|
| 1. Purpose of Tests. | Page | 1. |
| 2. Scope of Tests. | Page | 1. |

II. AVAILABLE DATA.

- | | | |
|---|------|----|
| 1. Brief Discussion of Previous
Tests and their Results. | Page | 3. |
|---|------|----|

III. MATERIALS, TEST-PIECES, AND METHOD OF TESTING.

- | | | |
|--------------------------------|------|----|
| 1. Description of Materials. | Page | 5. |
| 2. Description of Test-pieces. | Page | 5. |
| 3. Method of Testing. | Page | 6. |

IV. DATA OBTAINED.

- | | | |
|----------------------------|------|-----|
| 1. Tables Showing Results. | Page | 9. |
| 2. Curves Showing Results. | Page | 25. |

V. CONCLUSIONS.

- | | | |
|---|------|-----|
| 1. Discussion and Interpretation
of Results. | Page | 53. |
|---|------|-----|

I. INTRODUCTION.

1. Purpose of Tests.

In reinforced columns and beams for construction purposes, the bond between the concrete and the steel reinforcement is a very important factor. It is the purpose of these tests to investigate the strength of this bond under the different conditions under which reinforced concrete may be used in practice.

2. Scope of Tests.

These tests are not representative of practice, but comparative; that is, they do not give the actual bond stress which would be developed in practice, but they give the comparative strength under different conditions. They show how the bond is affected by the size of the reinforcing bar, the shape or form of this bar, the proportions in which the concrete is mixed, the age of the concrete, and the conditions of storage. Investigations of the effect of the size of the reinforcing bar were made by testing pieces containing respectively, 1/2-in., 5/8-in., 3/4-in., 1-in., and 1 1/4-in bars. Those for form of bar were made upon specimens containing the square corrugated bars known as the Johnson bars, and upon others containing the plain round bars. Tests showing the mixture of the concrete were made upon specimens containing the plain round bars, the concrete being mixed in the different proportions, 1-4-8, 1-3-6, 1-2-4, 1-1 1/2-3, and 1-1-2. Tests to show the effect of age of the

concrete were made upon specimens of 1-2-4 mixture, respectively, 2, 4, 7, 14, 30, and 60 days old, using both round and corrugated bars. Tests to show the effect of different conditions of storage were made upon specimens stored in sand, water, outside air, and air of laboratory.

In order to get the comparative quality of the concrete from each batch, compression cubes and cylinders were made. These cubes were 6-in. on the side and the cylinders were 8-in. in diameter and 16-in. long. In making the "mixture" specimens, three cubes and one cylinder were made from each of the different mixtures. In making the "time" specimens, three cubes were made from the first part of the batch, three from the middle portion, and three from the last. This was done on account of the size of the batch and the length of time required to make all of the specimens. In making the specimens for variation and size of bars, and storage specimens three cubes were made from each batch.

II. AVAILABLE DATA.

1. Brief Discussion of Previous Tests and Their Results.

Under this head it will not be the object to give the actual data of previous tests, but merely to state their general results and conclusions. It has been found that the resistance offered by a steel rod imbedded in concrete consists of frictional resistance, adhesion, and shear. When the concrete sets, it contracts and grips the imbedded bar. This grip causes frictional resistance to any force tending to pull out the bar. The force of adhesion is probably slight, and its effect in producing bond cannot be determined. Shear is the resistance offered by the shearing of the concrete, and probably comprises the greater part of the bond in the patent deformed bars. The shear caused by the irregularity of surface of a plain round bar is probably very slight.

From tests made in 1906 by Mr. Todd Kirk of the civil engineering class, it was found that a rich mixture gives higher bond resistance than a lean one, that the length of imbedment of the bar makes very little difference in the bond resistance up to the point where the stretch of steel does not cause uneven distribution of the bond stress along the length of the bar, and that flat bars give much lower resistance than round ones. In general, it was decided that the value of the bond resistance depends upon the smoothness of the surface of the bar, the uniformity of its diameter

and section, the adhesive strength of the concrete, and the grip developed in setting.

III. MATERIALS, TEST-PIECES, AND METHOD OF TESTING.

1. Materials.

In all of the tests made, the same kind and quality of cement, sand, and stone were used in making the specimens. The cement used was Chicago A-A portland. Results of tensile tests of this cement are given in Table IX. The sand was that known as torpedo sand, from Attica, Indiana, and was ordered screened through a 1/4-in. screen. The stone used was Kankakee limestone, and was ordered screened through a 1-in. screen and over a 1/4-in. screen. Table VIII shows the results of a number of tests to determine the fineness of the cement, sand, and stone. Tests were made to determine the per cent. of voids in the sand and stone, and the sand was found to contain about 28 per cent. of voids, and the stone about 50 per cent.

2. Test-pieces.

The form of test-piece used was a cylinder about 8 in. in diameter and 8 in. long, in the axis of which cylinder the steel reinforcing rod was placed. In making the test-pieces, two I-beams were set at the desired height, with their webs vertical and their inside flanges 2 or 3 in. apart. Flat iron plates were set upon the top flanges of these I-beams, and upon these were placed the circular galvanized iron forms, each form being kept from spreading by a circular iron band. The rods were placed inside these forms, their free ends between the I-beams, so that the

imbedded end of the rods projected $1/8$ to $1/4$ in. above the top of the forms. The concrete, after being mixed in the usual manner, great care being taken to secure a very thorough mixture, was put into the forms and carefully tamped. The specimens were allowed to remain in the forms for a period of 4 days, when the forms were removed and the specimens completely covered with damp sand. The compression cubes were treated in the same manner. The specimens which were tested for storage, were of course exceptions to this method of storage.

3. Method of Testing.

Previous to testing, all specimens more than 7 days old were set up on an iron plate in plaster of paris. This was allowed to harden for a day or so, in order to provide an even bearing on the plate at the bottom of the test specimen when it rests in the machine. In testing the specimen, a bearing block was placed upon the head of the machine and an iron plate placed upon this block. A rubber cushion was put between this plate and the plate upon the bottom of the test-piece, which was placed with the free end down. This free end was caught in the lower grip of the machine and an initial load of about 100 lb. applied. The instrument which was used to show the slip of the rod in the concrete, was then clamped upon the top of the specimen and the dial set to read zero. A speed of 0.05 in. per minute was then started and the loads observed for the point of beginning of slip, for each 0.001 in. up to 0.005 in., for each 0.005 in. up to 0.030 in., and for each 0.010 in. from then

on until the rod had completely slipped, or the specimen had failed in some other way.

On the following page is given a photograph showing a specimen placed in the machine ready to be tested. The instrument by means of which the readings were taken, is shown clamped to the top of the specimen.

TABLE NO. I.

Tests Showing Effect of Mixture Upon Bond Stress.

No. of Spec- imen	Mix- ture	Max. Load lb.	Stress lb. per sq. in.		In Steel at Max. Load	Run- ning Fric.	Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load			
202	1-4-8	5,370	173	283	12,100	210	0.025
203	1-4-8	5,100	200	270	11,600	210	0.020
204	1-4-8	4,270	145	226	9,700	130	0.025
205 Average	1-4-8	5,150	<u>161</u> 170	<u>273</u> 263	11,700	220	0.040
206	1-3-6	6,950	268	368	15,800	240	0.020
207	1-3-6	6,440	242	341	14,600	240	0.025
209 Average	1-3-6	6,660	<u>227</u> 246	<u>353</u> 354	15,100	240	0.025
221	1-2-4	7,730	295	410	17,500	290	0.015
222	1-2-4	5,310	215	282	12,100	210	0.010
223	1-2-4	5,260	194	278	11,900	240	0.025
224	1-2-4	8,240	282	437	18,700	320	0.025
225 Average	1-2-4	7,200	<u>299</u> 257	<u>382</u> 358	16,300	240	0.010
211	1-1 1/2-3	8,460	277	447	19,200	260	0.020
212	1-1 1/2-3	8,640	285	457	19,400	320	0.030
213	1-1 1/2-3	7,630	313	404	17,300	230	0.025
214	1-1 1/2-3	9,370	313	495	21,200	370	0.040
215 Average	1-1 1/2-3	7,590	<u>294</u> 297	<u>402</u> 441	17,200	260	0.030

TABLE NO. I. (Contd.)

No. of Spec- imen	Mix- ture	Max. Load lb.	Stress lb. per sq. in.				Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load	In Steel at Max. Load	Run- ning Fric.	
216	1-1-2	8,800	357	465	20,000	370	0.030
217	1-1-2	9,470	415	500	21,400	420	0.030
218	1-1-2	7,430	312	393	16,900	320	0.025
220	1-1-2	8,830	<u>342</u>	<u>467</u>	20,000	400	0.025
Average			356	456			

Note:-

Specimens used in above tests were 30 days old and contained 3/4-in. round rods. Area of rod = 0.442 sq. in. Surface in contact = 18.9 sq. in. Elastic limit of steel = 38,000 lb. per sq. in.

In all cases the failure was caused by the slipping of the rod.

TABLE NO. II.

Tests Showing Effect of Storage Upon Bond Stress.

No. of Spec- imen	Meth- od of Stor- age	Max. Load lb.	Stress lb. per sq. in.		In Steel at Max. Load	Run- ning Fric.	Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load			
301		6,600	273	350	15,000	250	0.010
302	In	6,370	278	338	14,400	250	0.010
303	water	7,340	330	389	16,600	250	0.010
305		6,620	<u>297</u>	<u>352</u>	15,000	250	0.015
Average			295	360			
306	Open	7,610	289	404	17,000	370	0.030
307	air of	8,140	334	432	18,400	380	0.020
308	labor-	7,200	285	382	16,300	350	0.030
309	atory	6,990	267	370	15,800	300	0.020
310		7,380	<u>308</u>	<u>392</u>	16,700	320	0.015
Average			317	396			
311		6,220	255	331	14,100	280	0.015
312	Out-	6,820	277	362	15,500	260	0.015
313	side	7,380	265	392	16,700	340	0.015
314	air	7,000	287	372	15,900	320	0.020
315		6,870	<u>273</u>	<u>364</u>	15,500	320	0.020
Average			271	364			
316		5,040	227	268	11,400	180	0.010
317	Damp	4,850	117	257	11,000	210	0.020
319		6,130	263	325	13,900	240	0.015
320	sand	6,760	284	358	15,300	270	0.015
321		7,120	<u>308</u>	<u>378</u>	16,100	300	0.015
Average			240	354			

TABLE NO. II. (Contd.)

Note:-

Specimens used in above tests were made from 1-2-4 concrete and contained 3/4-in. round rods. Age 30 days. Elastic limit of steel = 38,000 lb. per sq. in. Area of rod = 0.442 sq. in. Surface in contact = 18.9 sq. in.

In all cases the failure was caused by the slipping of the rod.

TABLE NO. III.

Tests Showing Effect of Variations in Size of Round
Rods Upon Bond Stress.

No. of Spec- imen	Diam. of Rod in.	Max. Load lb.	Stress lb. per sq. in.				Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load	In Steel at Max. Load	Run- ning Fric.	
251	1/2	6,530	398	520	33,200	350	0.015
253	1/2	5,480	351	436	27,800	270	0.010
254	1/2	6,060	369	482	30,800	350	0.015
255	1/2	6,100	<u>392</u>	<u>485</u>	31,000	300	0.010
Average			378	481			
256	5/8	7,160	368	457	23,300	320	0.015
257	5/8	6,770	350	432	22,000	300	0.010
258	5/8	9,420	498	601	30,700	300	0.010
260	5/8	6,120	<u>328</u>	<u>390</u>	19,900	400	0.015
Average			386	446			
221	3/4	7,730	295	410	17,500	290	0.015
222	3/4	5,310	215	282	12,000	210	0.010
223	3/4	6,260	194	278	14,200	240	0.025
224	3/4	8,240	282	437	18,700	320	0.025
225	3/4	7,200	<u>299</u>	<u>382</u>	16,300	240	0.010
Average			257	360			
262	1	13,020	393	518	16,600	370	0.020
263	1	12,900	417	513	16,500	400	0.020
264	1	14,000	432	560	17,900	420	0.015
265	1	9,720	<u>290</u>	<u>387</u>	12,400	300	0.020
Average			383	495			

TABLE NO. III. (Contd.)

No. of Spec- imen	Diam. of Rod in.	Max. Load lb.	Stress Bond at Slip of 0.001 in.	lb. At Max. Load	per sq. in.	In Steel at Max. Load	Run- ning Fric.	Slip at Max. Load in.
266	1 1/4	12,400	268	395		10,100	400	0.020
267	1 1/4	12,690	306	403		10,300	350	0.015
268	1 1/4	15,590	320	495		12,700	330	0.015
269	1 1/4	17,000	365	540		13,800	300	0.015
270	1 1/4	14,000	<u>328</u>	<u>445</u>		11,400	300	0.020
Average			<u>317</u>	<u>456</u>				

APPENDIX TO TABLE III.

Diameter of Rod in.	Area of Rod sq. in.	Surface in Contact sq. in.
1/2	0.197	12.6
5/8	0.308	15.7
3/4	0.442	18.9
1	0.786	25.2
1 1/4	1.230	31.5

Note:-

Specimens used in the above tests were 30 days old and were made of 1-2-4 concrete. Elastic limit of steel = 38,000 lb. per sq. in.

In all cases the failure was caused by the slipping of the rod.

TABLE NO. IV.

Tests Showing Effect of Age of Specimen Upon
Bond Stress.

No. of Spec- imen	Age in Days	Max. Load lb.	<u>Stress lb. per sq. in.</u>				Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load	In Steel at Max. Load	Run- ning Fric.	
101	2	1,150	41	61	2,600	50	0.015
103	2	1,490	44	79	3,380	80	0.025
104	2	1,120	37	59	2,550	80	0.015
105	2	1,000	<u>47</u>	<u>53</u>	2,270	50	0.010
Average			42	63			
111	4	3,200	90	169	7,250	130	0.015
112	4	2,400	88	127	5,450	100	0.015
113	4	2,690	92	142	6,100	120	0.020
114	4	2,290	90	121	5,200	70	0.010
115	4	2,510	<u>93</u>	<u>133</u>	5,700	100	0.015
Average			91	138			
121	7	3,610	109	192	8,200	130	0.020
122	7	2,750	88	145	6,240	100	0.015
123	7	2,760	86	146	6,280	100	0.020
124	7	4,290	143	227	9,760	180	0.025
125	7	2,570	<u>105</u>	<u>136</u>	5,820	100	0.010
Average			106	169			
131	14	4,110	178	217	9,320	150	0.015
132	14	6,100	202	322	13,850	250	0.020
133	14	5,130	170	272	11,600	220	0.020
134	14	5,740	207	304	13,000	240	0.025
135	14	4,170	<u>162</u>	<u>221</u>	9,460	150	0.015
Average			184	267			

TABLE NO. IV. (Contd.)

No. of Spec- imen	Age in Days	Max. Load lb.	Stress lb. per sq. in.				Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load	In Steel at Max. Load	Run- ning Fric.	
141	30	7,730	295	410	17,600	300	0.015
142	30	5,310	215	282	12,100	200	0.010
143	30	5,260	194	278	11,900	230	0.025
144	30	8,240	282	437	18,700	320	0.025
145	30	7,200	<u>299</u>	<u>382</u>	16,300	200	0.010
Average			<u>257</u>	<u>360</u>			
151	60	12,040	492	638	25,000	450	0.015
152	60	12,310	485	651	28,000	450	0.015
153	60	11,070	405	585	25,000	400	0.015
154	60	7,560	<u>307</u>	<u>400</u>	17,200	300	0.020
Average			<u>422</u>	<u>568</u>			

Note:-

Specimens used in the above test were made of 1-2-4 concrete and contained 3/4-in. round rods. Area of rod = 0.442 sq. in. Surface in contact = 18.9 sq. in. Elastic limit of steel = 38,000 lb. per sq. in.

In all cases the failure was caused by the slipping of the rod.

TABLE NO. V.

Tests Showing Effect of Age of Specimen Upon
Bond Stress.

No. of Spec- imen	Age in Days	Max. Load lb.	Stress lb. per sq. in.				Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load	In Steel at Max. Load	Run- ning Fric.	
106	2	1,510	35	95	6,050	70	0.050
107	2	1,370	36	86	5,500	70	0.050
108	2	1,560	54	98	6,250	70	0.030
109	2	2,190	48	138	8,780	100	0.025
110	2	1,930	<u>58</u>	<u>121</u>	7,700	70	0.020
Average			46	107			
116	4	4,200	102	263	16,800	200	0.040
117	4	3,310	85	208	13,200	110	0.030
118	4	3,670	104	230	14,700	200	0.040
119	4	3,430	101	215	13,700	200	0.050
120	4	3,500	<u>78</u>	<u>219</u>	14,000	200	0.060
Average			94	227			
127	7	4,490	121	281	17,900	220	0.050
128	7	3,870	119	242	15,500	180	0.025
129	7	4,670	124	294	18,300	250	0.040
130	7	5,240	<u>129</u>	<u>328</u>	21,000	280	0.040
Average			123	286			
138	14	6,960	273	435	27,800	200	0.020
139	14	8,000	218	500	32,000	400	0.050
140	14	7,550	<u>227</u>	<u>472</u>	30,200	400	0.030
Average			239	469			

TABLE NO. V. (Contd.)

No. of Spec- imen	Age in Days	Max. Load lb.	Stress lb. per sq. in.				Slip at Max. Load in.
			Bond at Slip of 0.001 in.	At Max. Load	In Steel at Max. Load	Run- ning Fric.	
146	30	10,390	283	650	41,500	600	0.110
147	30	9,460	325	592	37,800	600	0.080
148	30	10,210	366	652	41,000	600	0.100
149	30	10,520	313	659	42,000	600	0.100
150	30	10,650	<u>287</u>	<u>667</u>	42,600	600	0.120
Average			315	644			
156	60	26,000	610	1,086	46,300		0.015
157	60	17,000	522	708	30,300		0.010
158	60	25,000	518	1,042	44,500		0.025
159	60	21,720	<u>395</u>	<u>908</u>	38,500		0.030
Average			511	937			

Note:-

Specimens in the above tests were made of 1-2-4 concrete and contained 1/2-in. corrugated bars, with the exception of the 60 day specimens which contained 3/4-in. rods. Area of 1/2-in. rod = 0.25 sq. in. Surface in contact = 16.0 sq. in. Elastic limit of steel = 55,000 lb. per sq. in. Area of 3/4-in. rod = 0.562 sq. in. Surface in contact = 24.0 sq. in. Elastic limit of steel = 53,500 lb. per sq. in.

In all cases the failure was caused by the splitting of the concrete.

TABLE NO. VI.

Summary of Tests.

No. of Tests	Mix- ture	Meth- od of Stor- age	Form and Size of Bar	Age of Spec- imen at Time of Test Days	Aver- age Max. Bond Stress lb. per sq. in.	Percent Increase over Preceding Set of Tests	Percent Increase over 1-4-8 Mixture
--------------------	--------------	--------------------------------	----------------------------------	---	--	---	---

Variation in Mixture.

4	1-4-8			30	263		
4	1-3-6	Damp	3/4-in.	30	303	15	15
5	1-2-4			30	360	18	36
5	1-1 1/2-3	sand	round	30	441	23	68
4	1-1-2			30	456	3	74

Variation in Method of Storage.

4	1-2-4	Water		30	357		
5	1-2-4	Air of lab.	3/4-in.	30	396		
5	1-2-4	Outside air.		30	364		
5	1-2-4	Damp sand.	round	30	354		

Variation in Size of Bar.

4	1-2-4		1/2-in. round	30	481		
4	1-2-4	Damp	5/8-in. round	30	446		
5	1-2-4		3/4-in. round	30	360		
4	1-2-4	sand	1-in. round	30	494		
5	1-2-4		1 1/4-in. round	30	456		

TABLE NO. VI. (Contd.)

No. of Tests	Mix- ture	Meth- od of Stor- age	Form and Size of Bar	Age of Spec- imen at Time of Test Days	Aver- age Max. Bond Stress lb. per sq. in.	Percent Increase over Preceding Set of Tests	Percent Increase over 2 day Tests
--------------------	--------------	--------------------------------	----------------------------------	---	--	---	---

Variation in Age of Concrete.

3	1-2-4			2	63		
5	1-2-4	Damp	3/4-in.	4	138	119	119
5	1-2-4			7	169	23	169
5	1-2-4	sand	round	14	267	58	324
5	1-2-4			30	360	34	468
4	1-2-4			60	568	59	800
4	1-2-4			2	107		
5	1-2-4	Damp	1/2-in.	4	227	75	75
4	1-2-4			7	286	26	167
3	1-2-4	sand	corru-	14	469	64	338
4	1-2-4		gated	30	644	37	500
4	1-2-4		3/4-in. corrug.	60	937	46	778

TABLE NO. VII.

Cube and Cylinder Tests Made in Connection
with Bond Tests.

No. of Spec- imen	Mix- ture	Max. Load	Unit Load lb. per sq. in.	Aver- age Unit Load lb. per sq. in.	Remarks
201	1-4-8	21,500	430		
206	1-3-6	24,100	480		
211	1-1 1/2-3	63,200	1,260		
216	1-1-2	116,500	2,320		These specimens made from batch of concrete from which specimens for "mixture" were made.
201-1	1-4-8	31,700	880		
201-2	1-4-8	33,200	922	909	
201-3	1-4-8	33,250	924		
206-1	1-3-6	33,450	930		
206-2	1-3-6	32,200	895	895	All specimens tes- ted at 30 days.
206-3	1-3-6	30,900	860		
211-1	1-1 1/2-3	67,400	1,870		
211-2	1-1 1/2-3	62,800	1,742	1,812	
211-3	1-1 1/2-3	66,800	1,825		
216-1	1-1-2	98,800	2,740		
216-2	1-1-2	85,600	2,380	2,617	
216-3	1-1-2	94,700	2,730		

Note:-

Specimens 201, 206, 211, and 216 were cylinders,
the remaining specimens were cubes.

TABLE NO. VII. (Contd.)

No. of Spec- imen	Max. Load	Unit Load lb. per sq. in.	Average Unit Load lb. per sq. in.	Remarks
Cubes made from the same batch of concrete as "time" specimens.				
100-1	51,200	1,420		
100-1	49,400	1,370	1,370	From first part of batch.
100-1	47,600	1,320		
100-2	50,000	1,390		
100-2	57,300	1,590	1,507	From middle portion of batch.
100-2	55,500	1,540		
100-3	53,700	1,490		
100-3	56,100	1,560	1,540	From last of batch.
100-3	56,500	1,500		
Cubes made from same batch of concrete as "storage" specimens.				
300-1	51,700	1,435		
300-2	57,700	1,600	1,432	
300-3	45,400	1,260		
Cubes made from same batch of concrete as "size" of bar specimens.				
250-1	44,600	1,340		
250-2	49,600	1,380	1,263	
250-3	42,000	1,170		

TABLE NO. VIII.

Fineness of Chicago A. A. Cement.

Test	Percent passing seive No.		
No.	75	100	200
1	99.3	98.5	90.1
2	98.5	95.6	81.5
3	98.0	94.5	79.7
Average	98.6	96.2	83.7

Fineness of Torpedo Sand.

Percent passing seive No.										
3	5	10	12	16	18	30	40	50	74	150
99.2	89.0	64.7	57.8	49.9	39.0	21.6	11.8	5.1	2.6	0.46

Note:-

These values are the averages of 21 tests from samples taken at intervals throughout the season.

Fineness of Kankakee Limestone.

Percent passing seive No.						
1-in.	3/4-in.	1/2-in.	3/8-in.	3	5	10
100	89.2	54.7	32.8	16.9	4.1	2.5

Note:-

These values are the averages of 8 tests from samples taken at intervals throughout the season.

TABLE NO. IX.

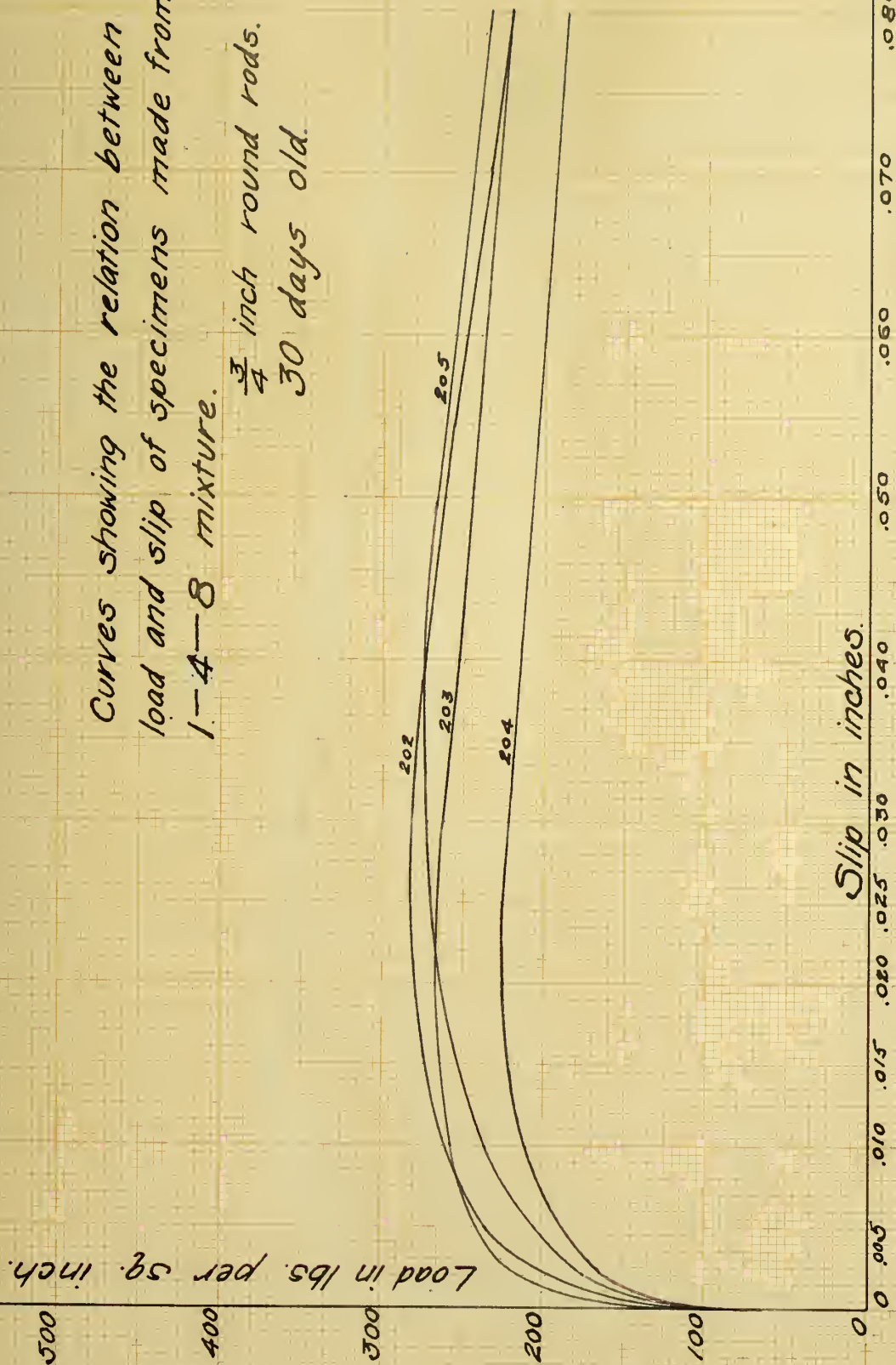
Tensile Strength of Chicago A. A. Cement.

Ref.	Tensile strength lb. per sq. in.			
No.	Age 7 days		Age 28 days	
	neat	1-3	neat	1-3
1	559	145	707	247
2	732	205	857	318
3	665	175	779	266
4	811	227	833	307
5	666	182	792	284
Average	686	184	794	284

Note:-

Each value given is average of tests from
5 briquettes.

Curves showing the relation between
load and slip of specimens made from
1-4-8 mixture.
 $\frac{3}{4}$ inch round rods.
30 days old.

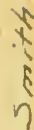


Curves showing the relation between
load and slip of specimens made from
1-3-6 mixture.
 $\frac{3}{4}$ inch round rods.
30 days old.

Load in lbs. per sq. inch.

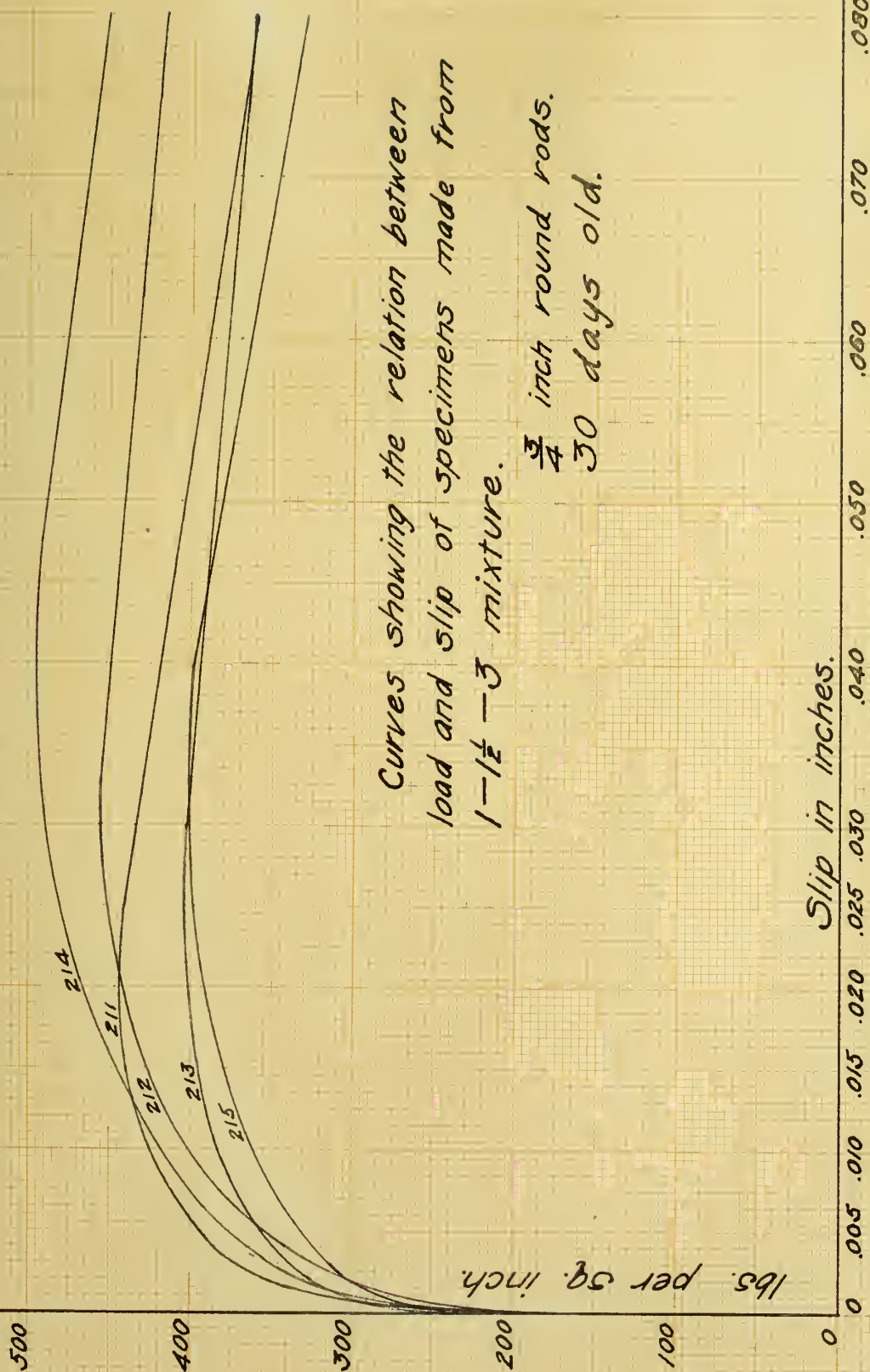
Slip in inches.





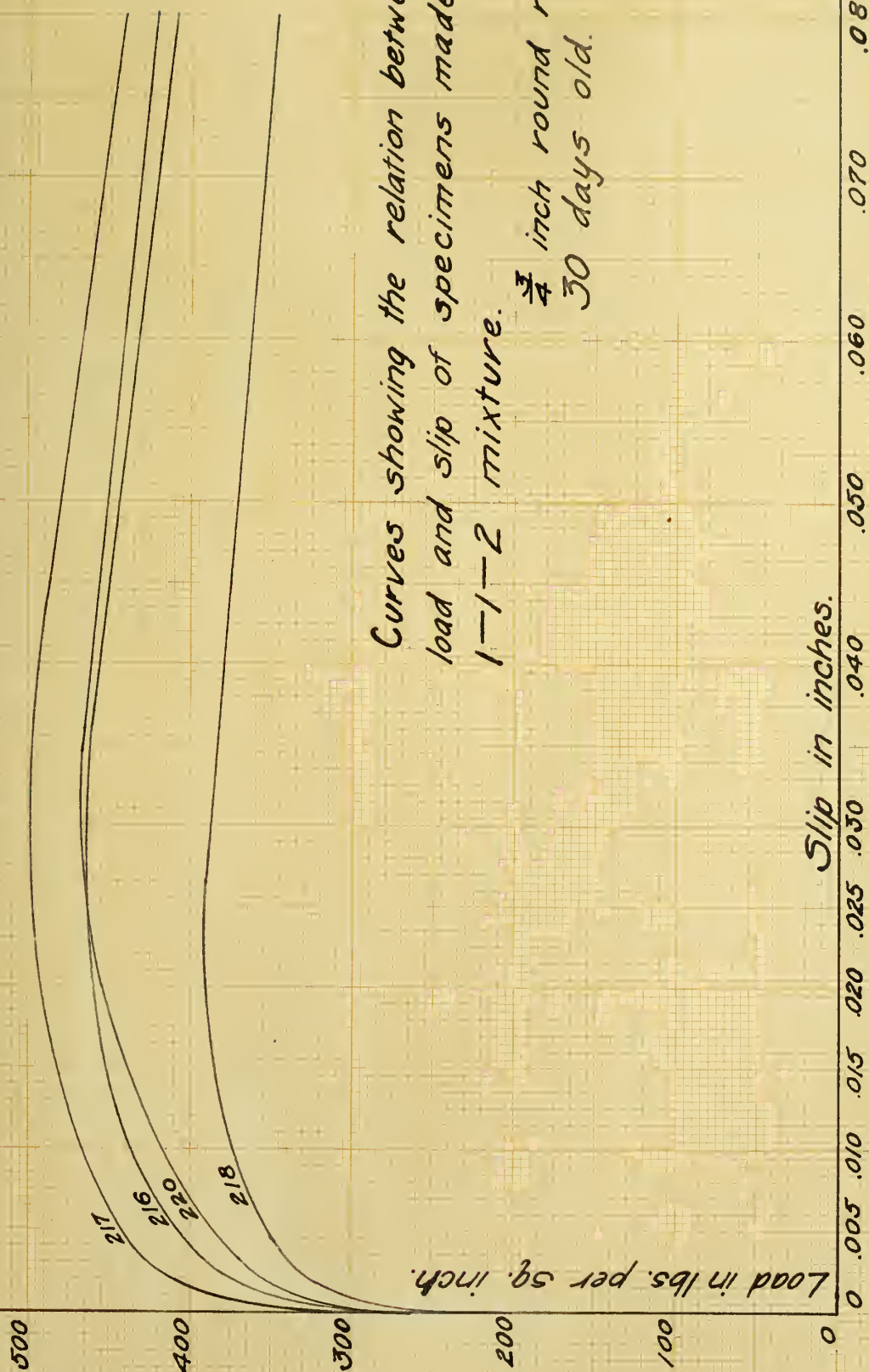
Curves showing the relation between load and slip of specimens made from 1-2-4 mixture.

$\frac{3}{4}$ inch round rods.
30 days old.



Curves showing the relation between
load and slip of specimens made from
 $1-1\frac{1}{2}-3$ mixture.

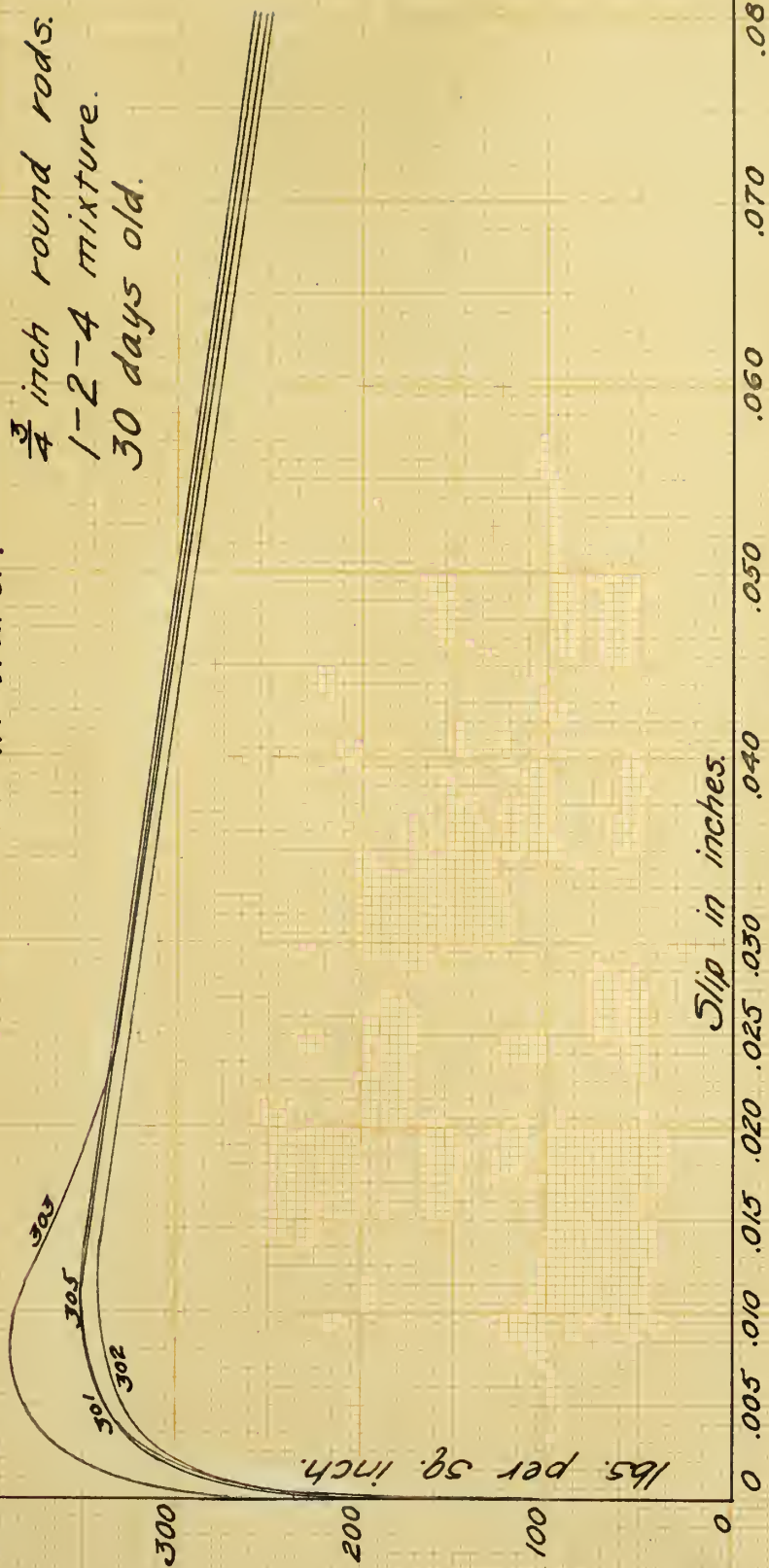
$\frac{3}{4}$ inch round rods.
30 days old.

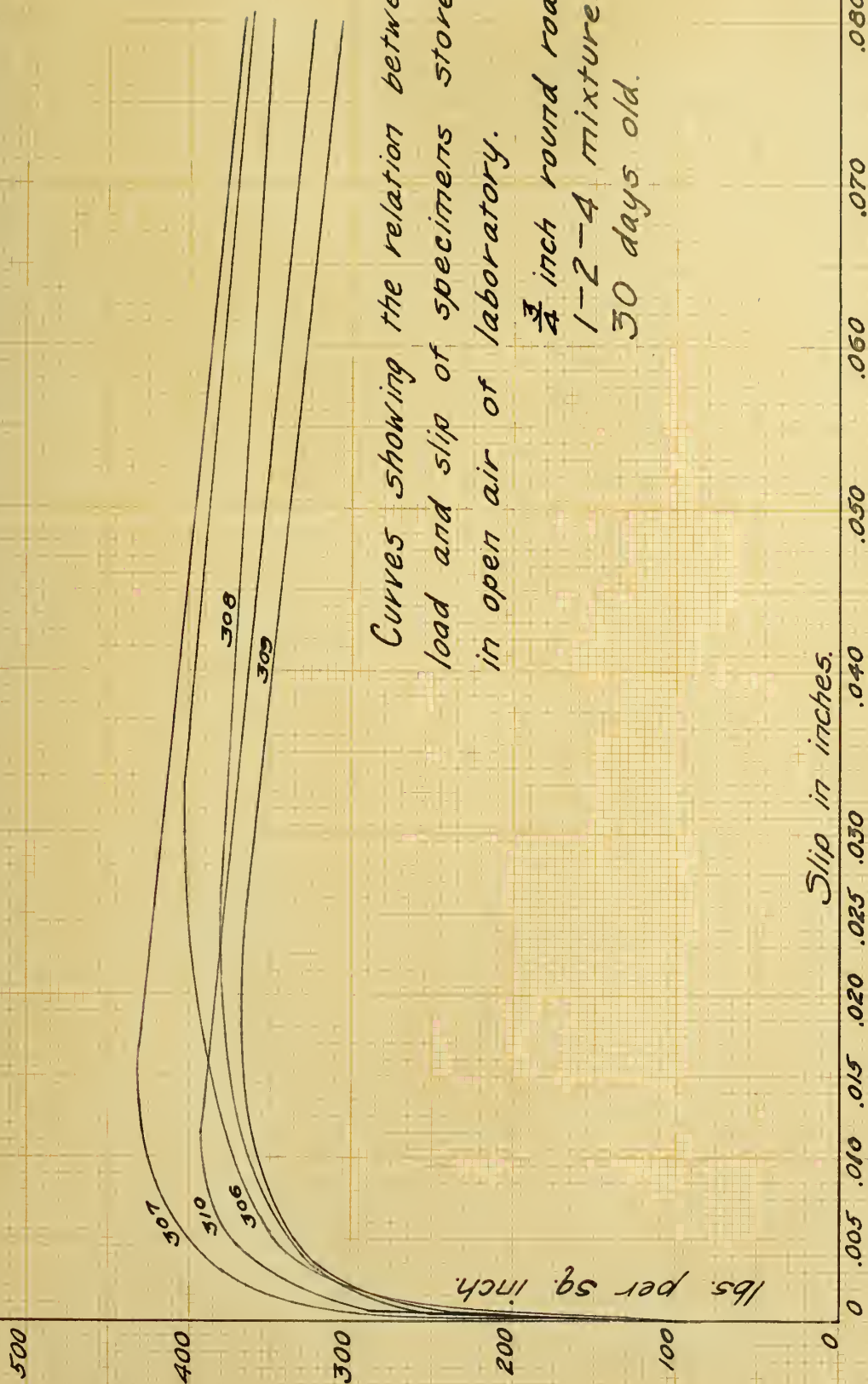


Curves showing the relation between
load and slip of specimens made from
1-1-2 mixture. $\frac{3}{4}$ inch round rods.
30 days old.

Curves showing the relation between
load and slip of specimens stored
in water.

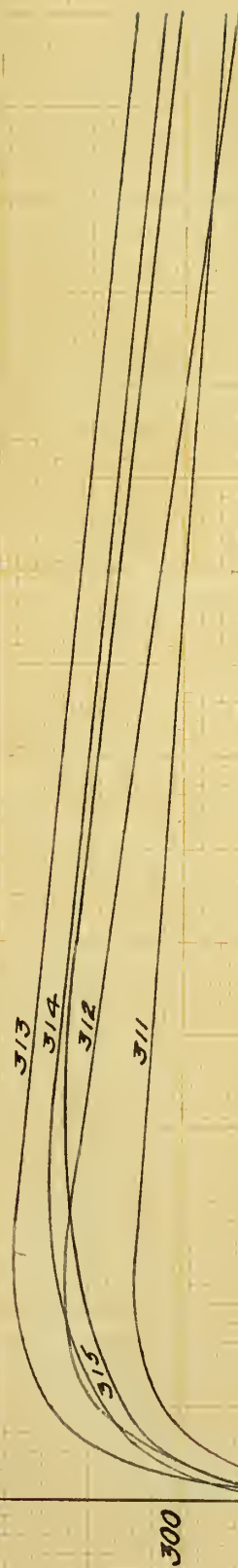
$\frac{3}{4}$ inch round rods.
1-2-4 mixture.
30 days old.





Curves showing the relation between load and slip of specimens stored in open air of laboratory.

$\frac{3}{4}$ inch round rods.
1-2-4 mixture.
30 days old.



Curves showing the relation between load and slip of specimens stored in outside air.

$\frac{3}{4}$ inch round rods.
1-2-4 mixture.
30 days old.

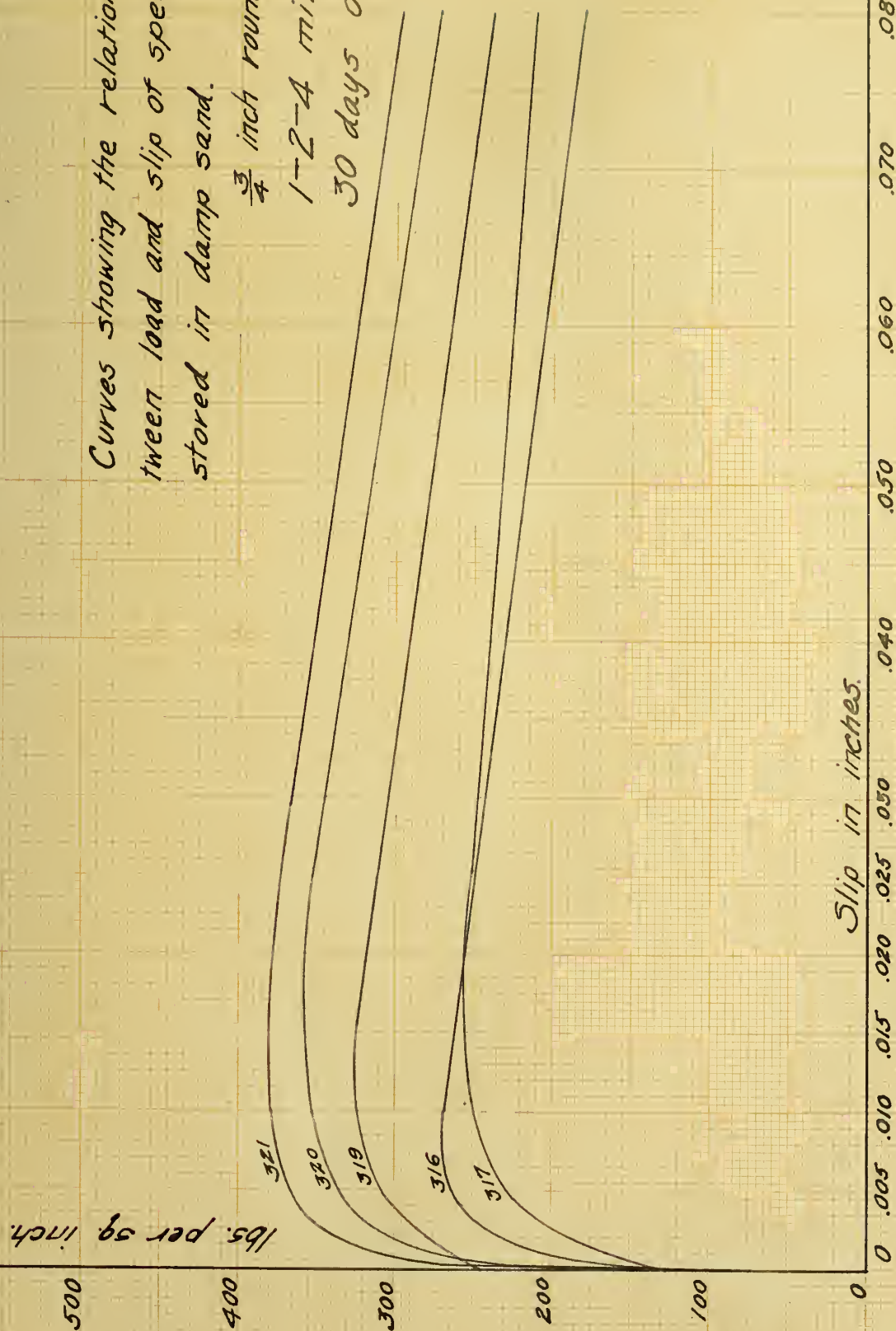
PLATE 9.

Curves showing the relation between load and slip of specimens stored in damp sand.

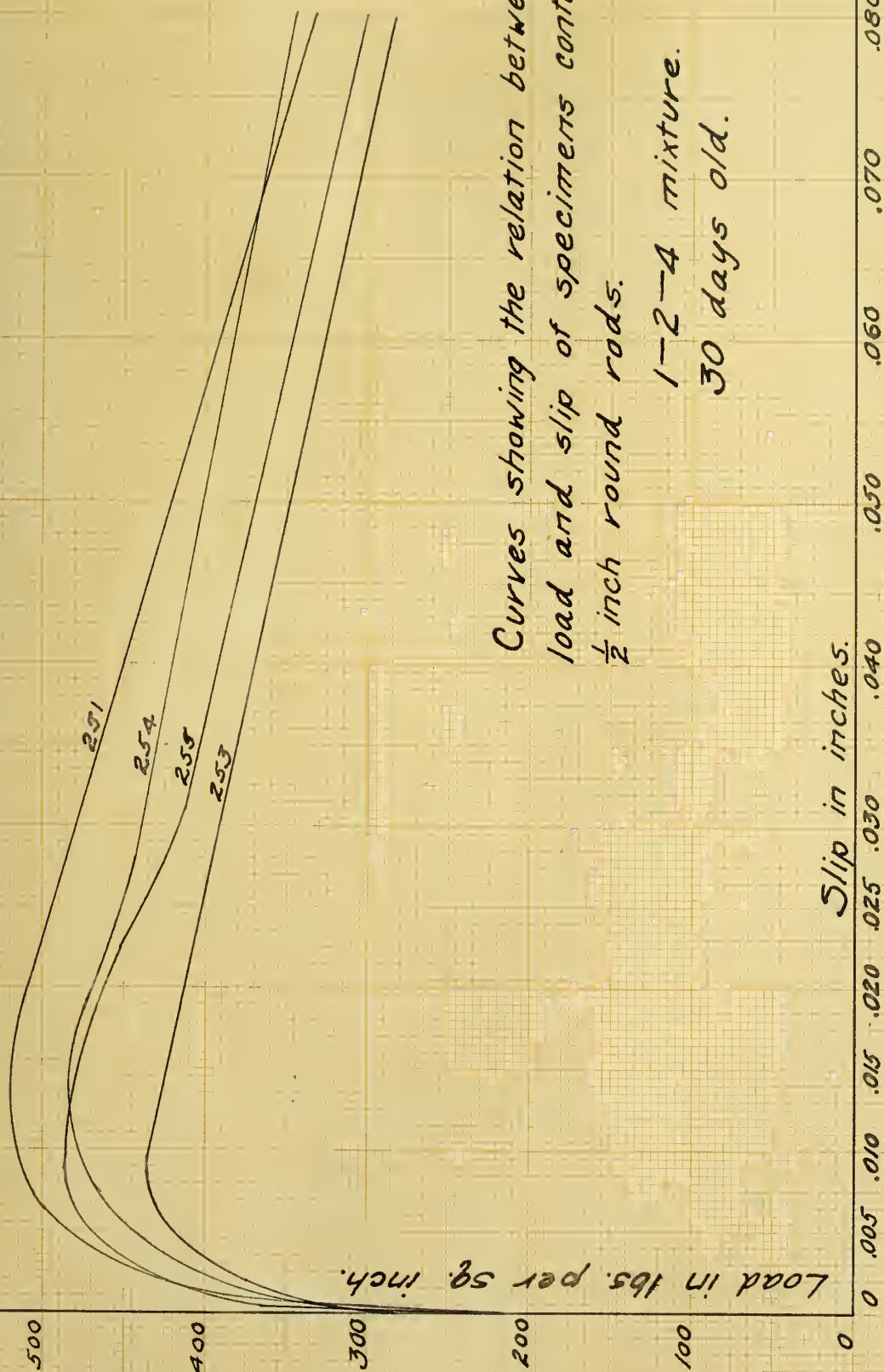
$\frac{3}{4}$ inch round rods.

1-2-4 mixture.

30 days old.

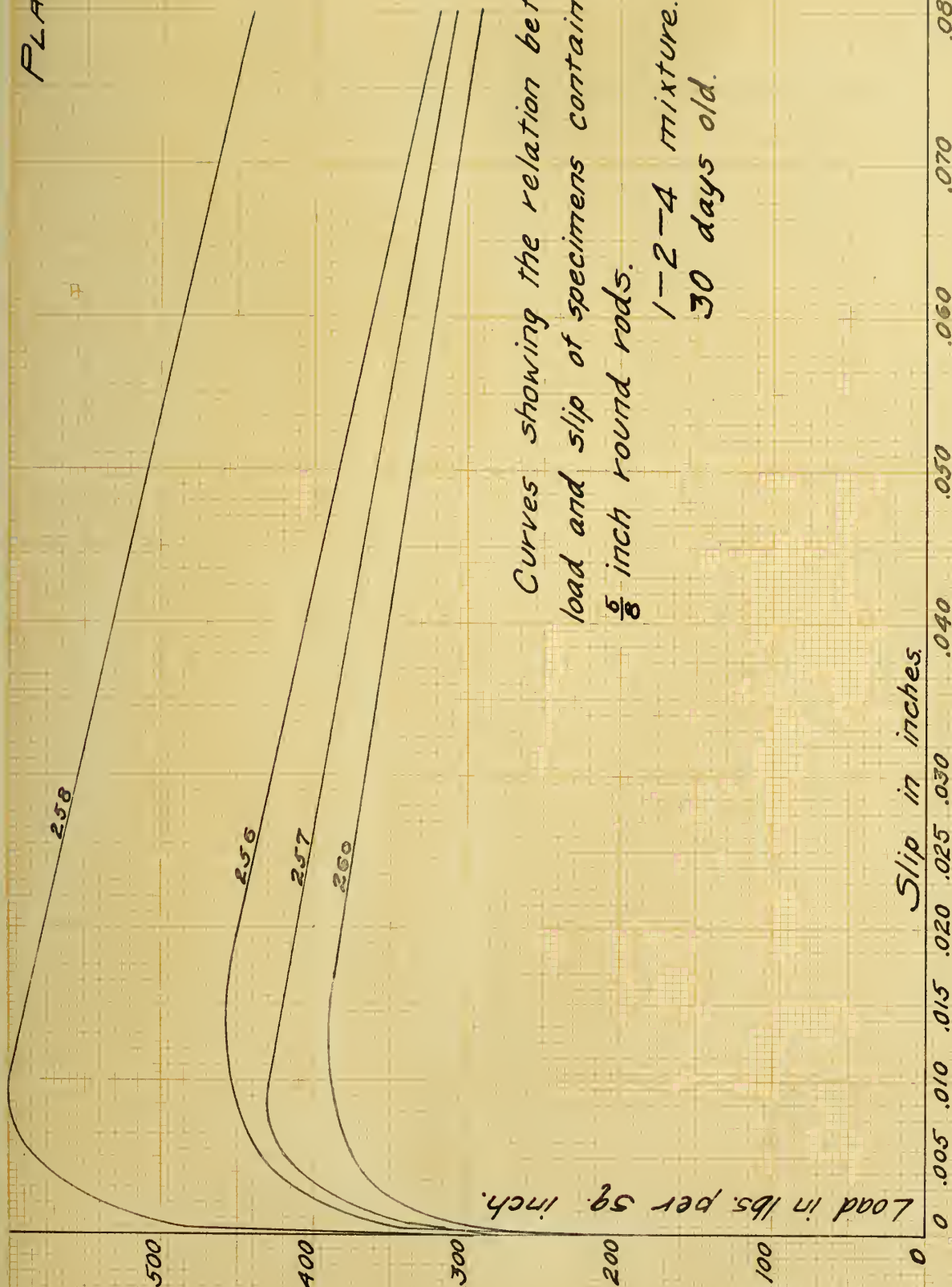


Smith



Curves showing the relation between
load and slip of specimens containing
 $\frac{1}{2}$ inch round rods.

1-2-4 mixture.
30 days old.



Curves showing the relation between
load and slip of specimens containing
 $\frac{5}{8}$ inch round rods.
1-2-4 mixture.
30 days old.

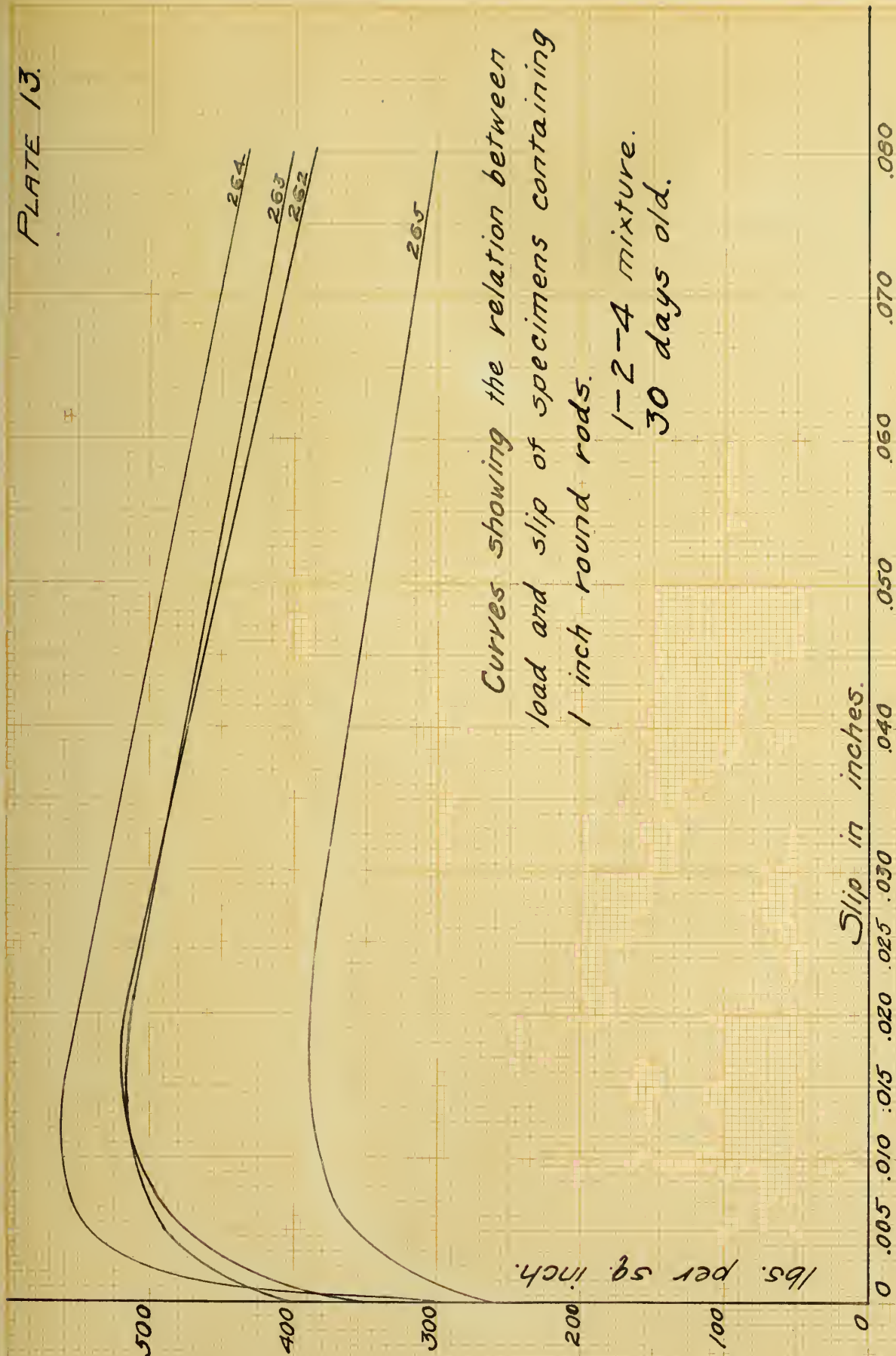
lbs. per sq. inch.



Curves showing the relation between
load and slip of specimens containing
 $\frac{3}{4}$ inch round rods.
1-2-4 mixture.
30 days old.

Slip in inches.

0 0.005 0.010 0.015 0.020 0.025 0.030 0.040 0.050 0.060 0.070 0.080



Curves showing the relation between
load and slip of specimens containing
1 inch round rods.

1-2-4 mixture.
30 days old.



Curves showing the relation between
load and slip of specimens containing
 $1\frac{1}{4}$ inch round rods.
1-2-4 mixture.
30 days old.

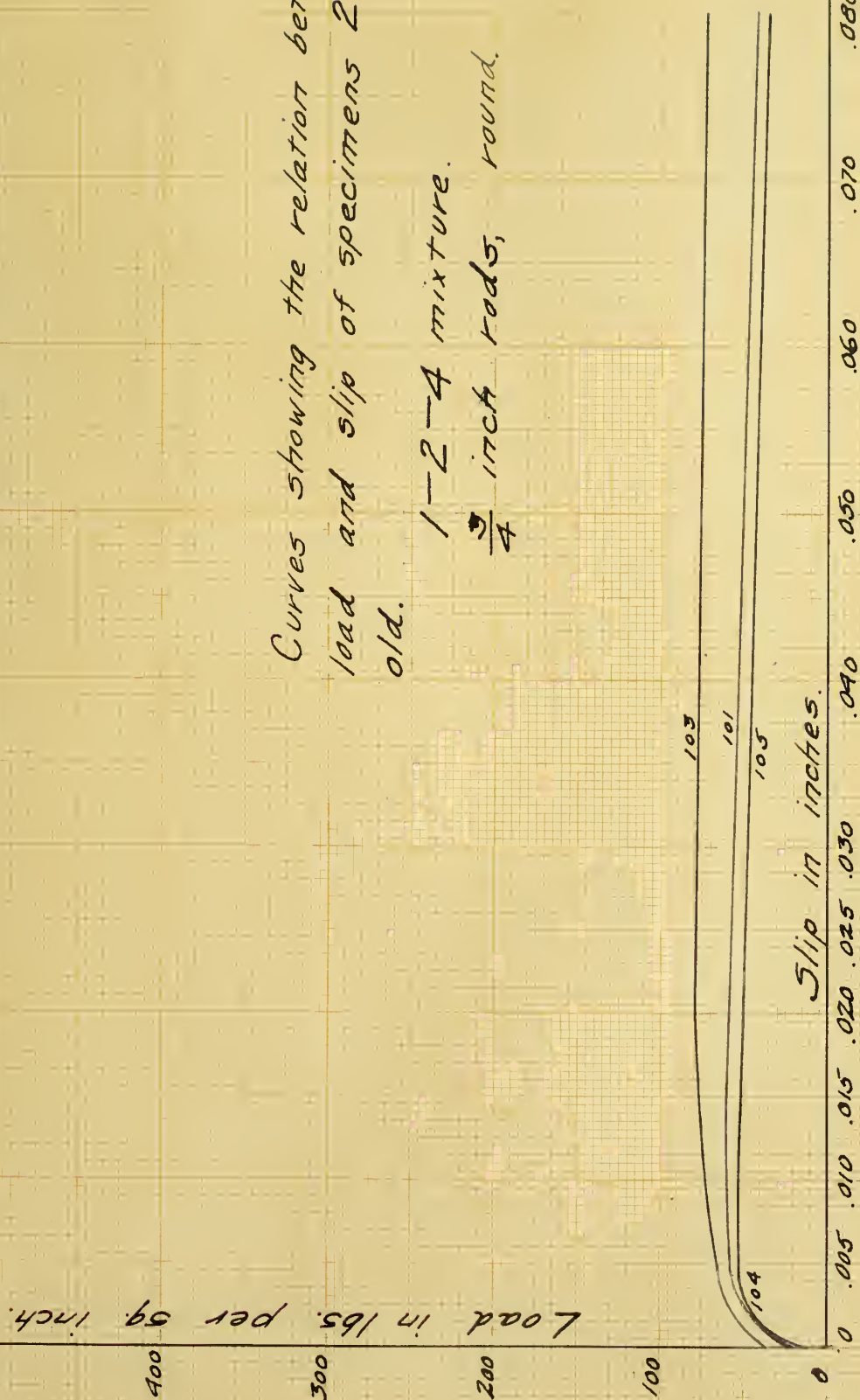
Slip in inches.

lbs. per sq. inch.

Curves showing the relation between load and slip of specimens 2 days old.

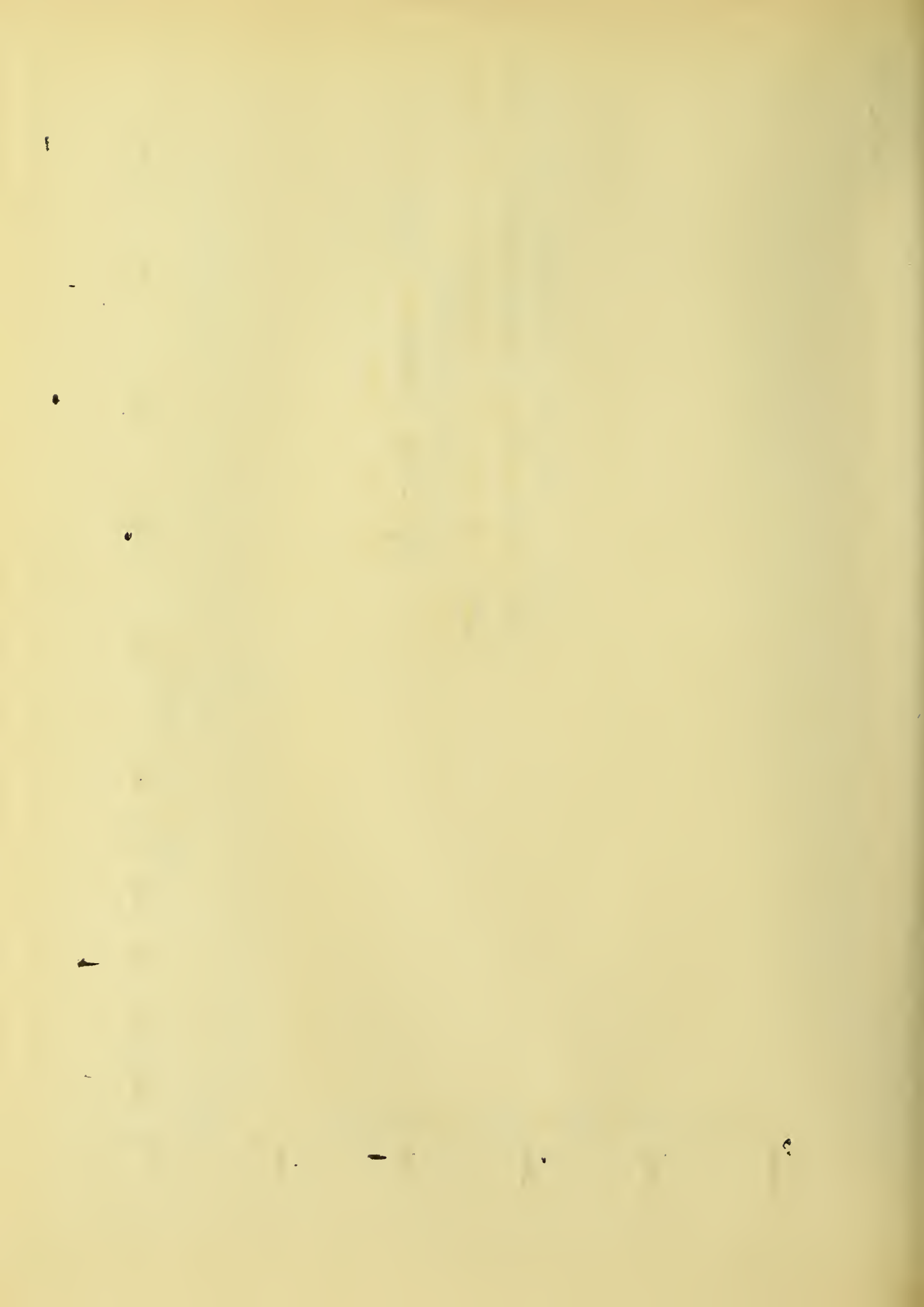
1-2-4 mixture.

$\frac{5}{4}$ inch rods, round.



Slip in inches.

Smith



Load in lbs. per sq. inch.

500

400

300

200

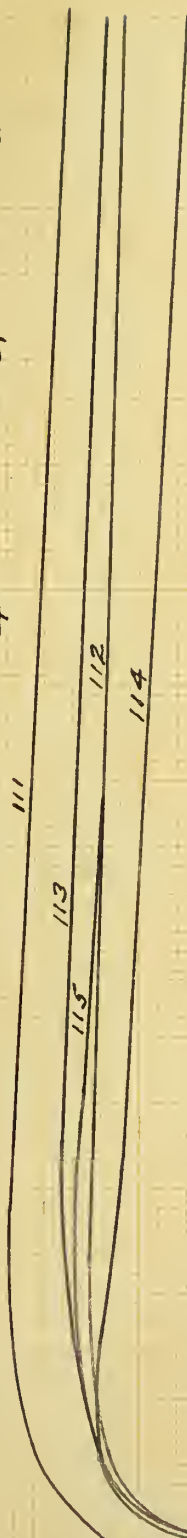
100

0

Curves showing the relation between
load and slip of specimens 4 days
old.

1-2-4 mixture.

$\frac{3}{4}$ inch rods, round.



Slip in inches.

0.080

0.070

0.060

0.050

0.040

0.030

0.025

0.020

0.015

0.010

0

Curves showing the relation between
load and slip of specimens 7 days
old.

1-2-4 mixture.

$\frac{3}{4}$ inch rods, round.

Load in lbs. per sq. inch.

Slip in inches.

500

400

300

200

100

0

124

121

123

125

122

.030

.025

.020

.015

.010

.005

0

.040

.050

.060

.070

.080

lbs. per sq. inch.

500
400
300
200
100
0

Curves showing the relation between
load and slip of specimens 14 days old.
1-2-4 mixture.
 $\frac{3}{4}$ inch rods, round.

132
134
133
135
131

Slip in inches.

0 .005 .010 .015 .020 .025 .030 .040 .050 .060 .070 .080



lbs. per sq. inch.

500

400

300

200

100

0



Curves showing the relation between
load and slip of specimens 30 days
old.

1-2-4 mixture.
 $\frac{3}{4}$ inch rods, round.

Slip in inches.

0.080

0.070

0.060

0.050

0.040

0.030

0.025

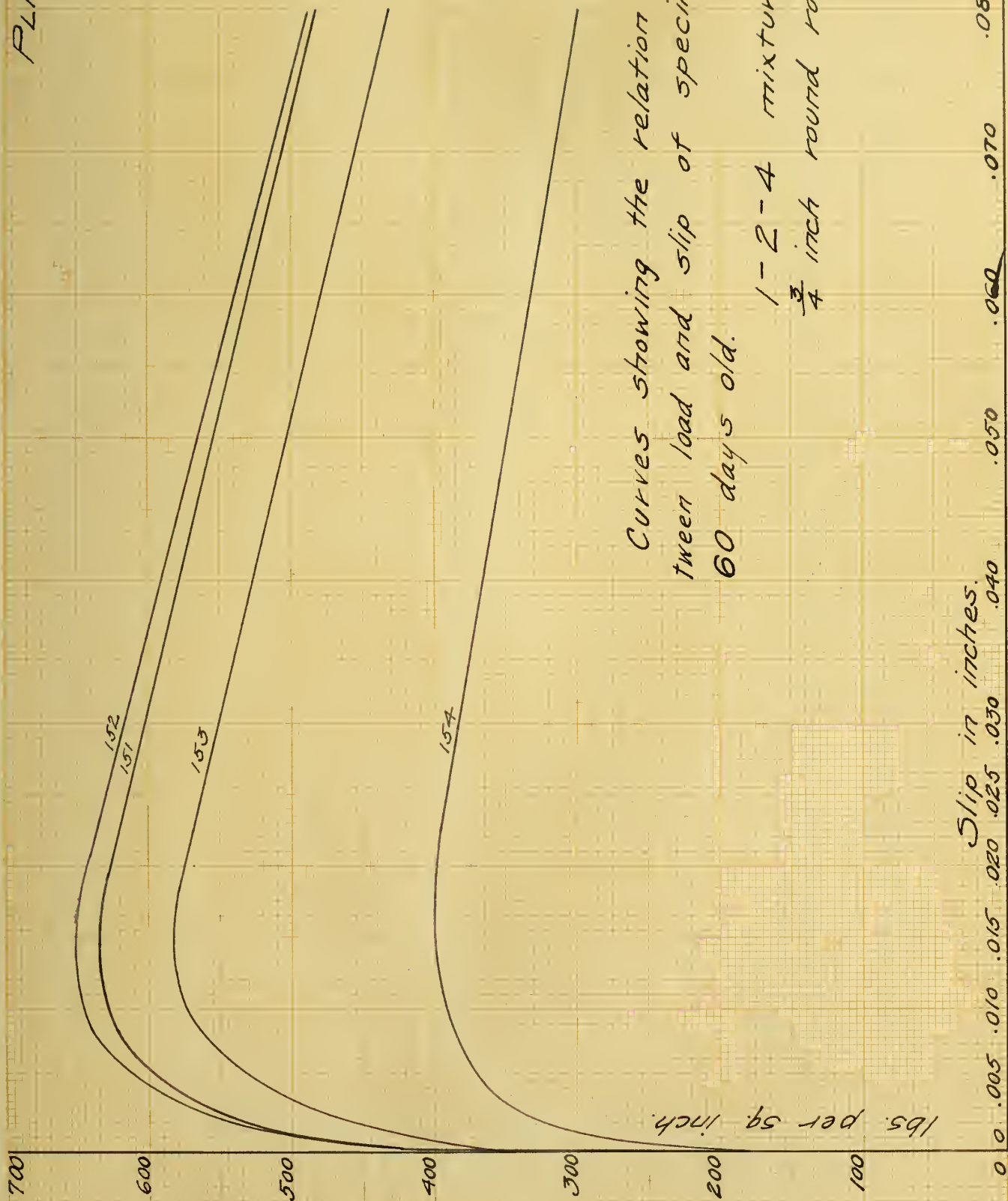
0.020

0.015

0.010

0.005

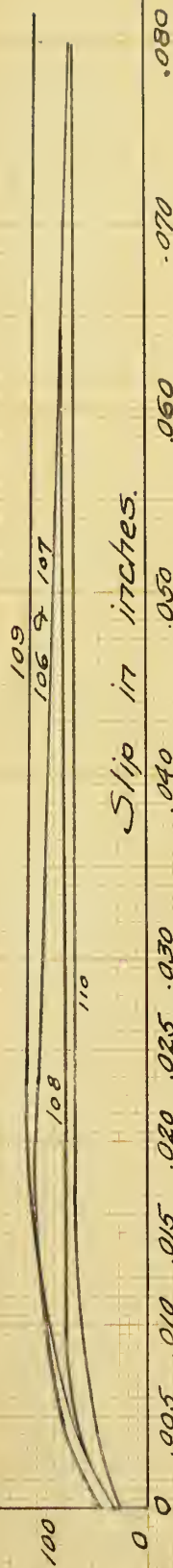
0



Load in lbs per sq. inch.

1000
900
800
700
600
500
400
300
200
100
0

Curves showing the relation between
load and slip of specimens 2 days
old.
1-2-4 mixture.
 $\frac{1}{2}$ inch corrugated
rods.

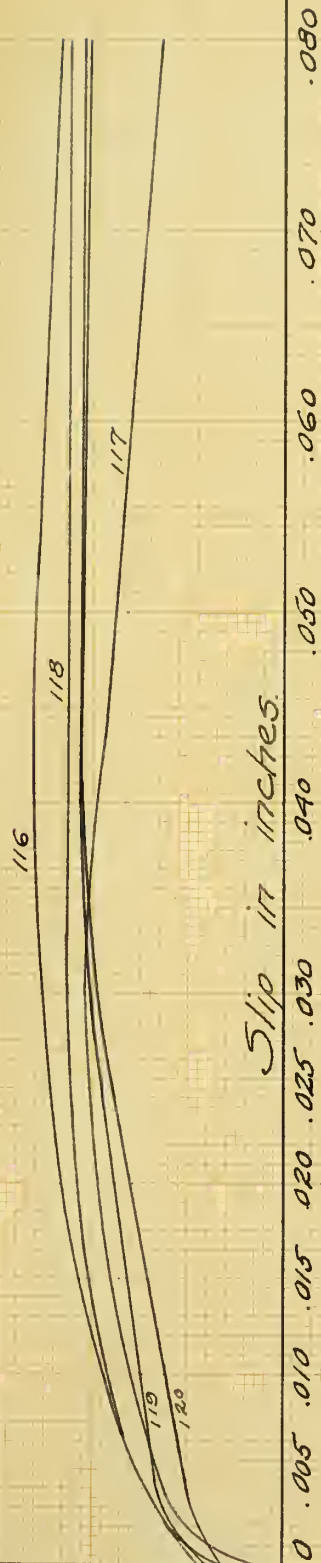


Slip in inches.

Curves showing the relation between
load and slip of specimens 4 days
old.
1-2-4 mixture.
 $\frac{1}{2}$ inch corrugated
rods.

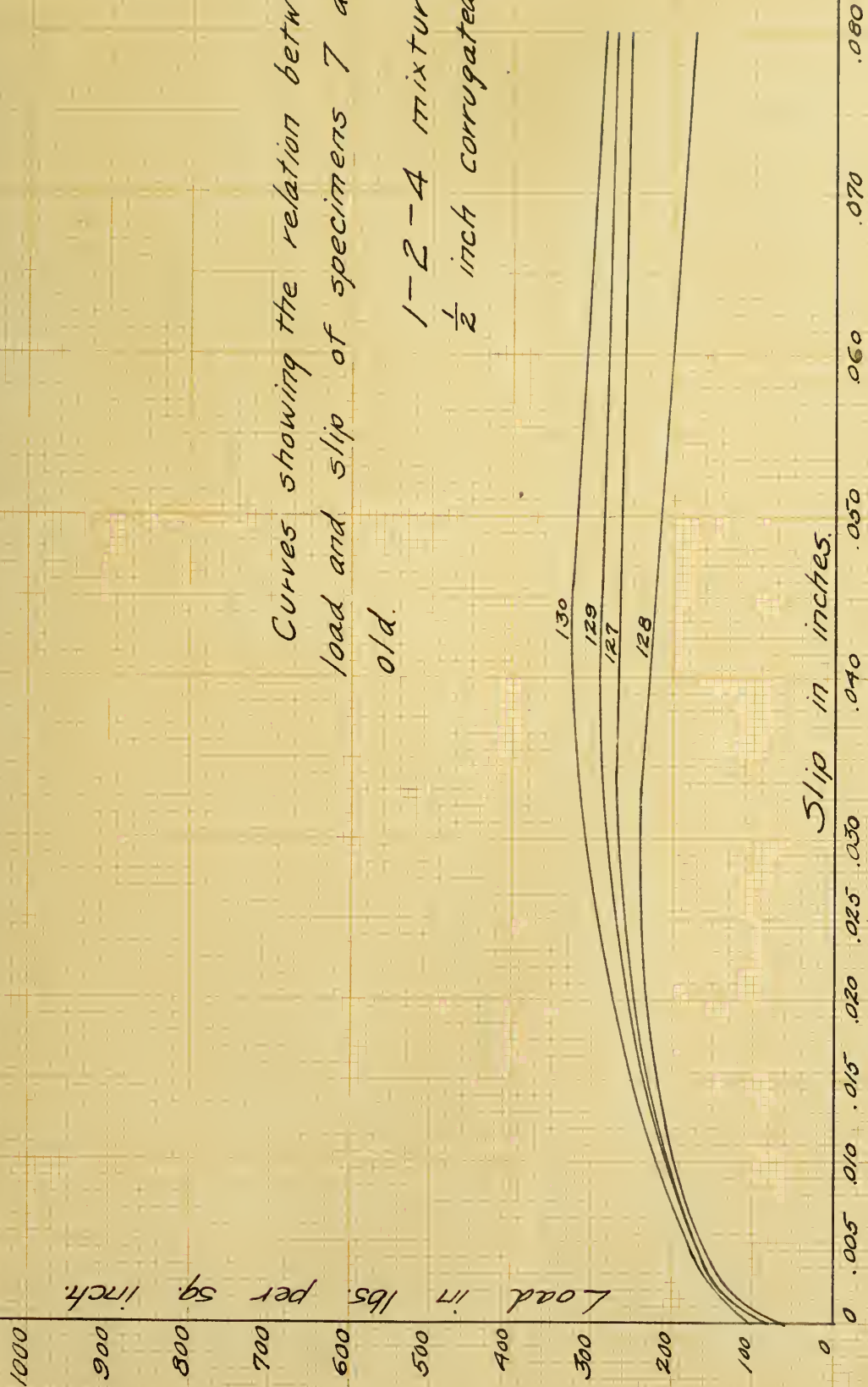
Load in lbs. per sq. inch

1000
900
800
700
600
500
400
300
200
100
0



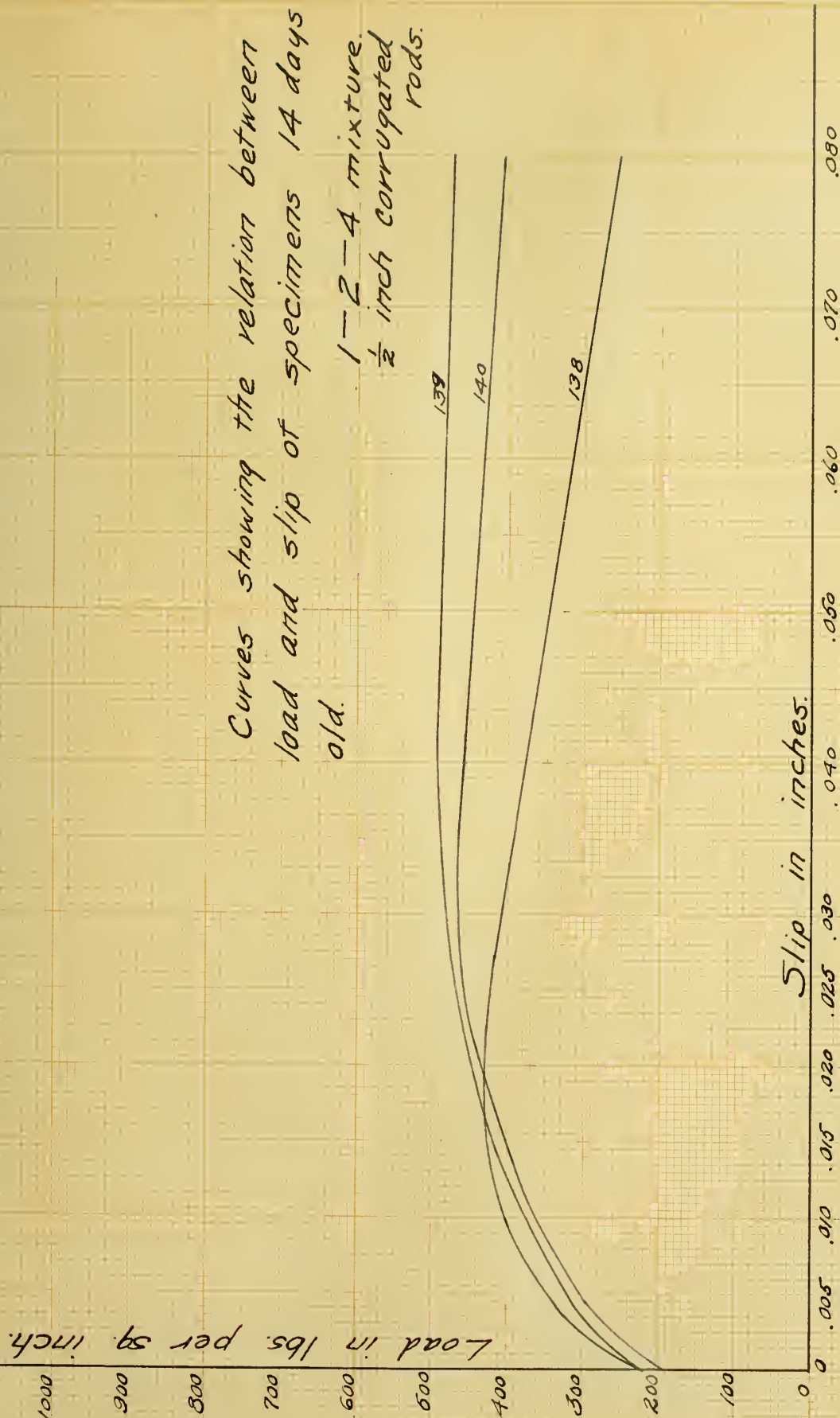
Curves showing the relation between
load and slip of specimens 7 days
old.

1-2-4 mixture.
 $\frac{1}{2}$ inch corrugated rods.



Smith

Curves showing the relation between
load and slip of specimens 14 days
old.
1-2-4 mixture.
 $\frac{1}{2}$ inch corrugated
rods.



Load in lbs. per sq. inch.

1000

900

800

700

600

500

400

300

200

100

0

149

146

147

148

Curves showing the relation between
load and slip of specimens 30 days
old.

1-2-4 mixture.

$\frac{1}{2}$ inch corrugated rods.

Slip in inches.

0

.005

.010

.015

.020

.025

.030

.035

.040

.045

.050

.055

.060

.065

.070

.075

.080

.085

.090

.095

.100

.105

.110

.115

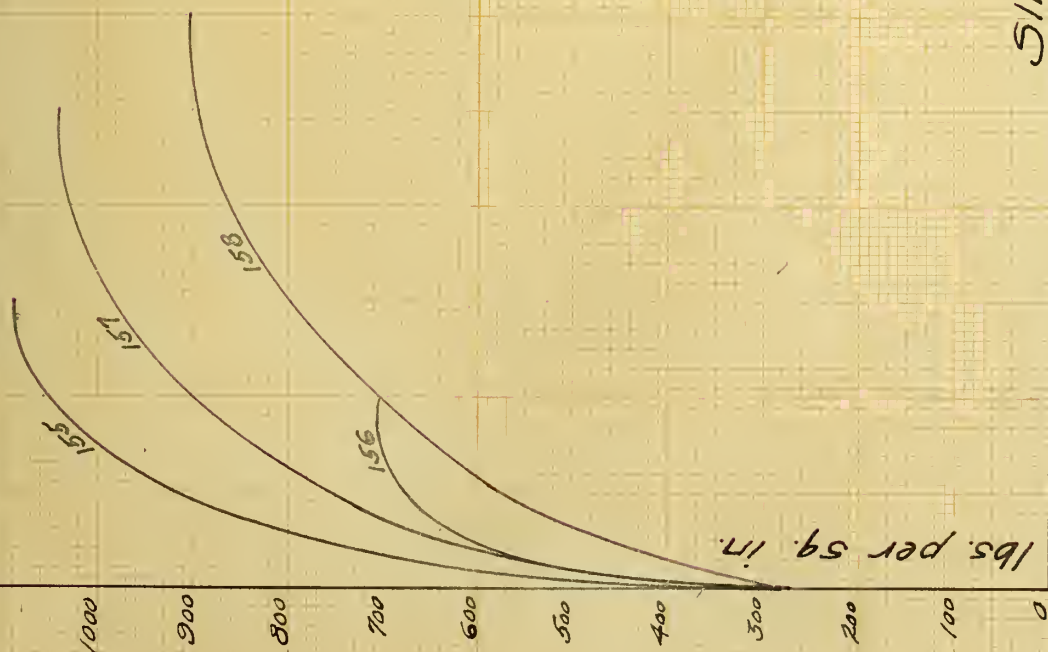
.120

.125

.130

.135

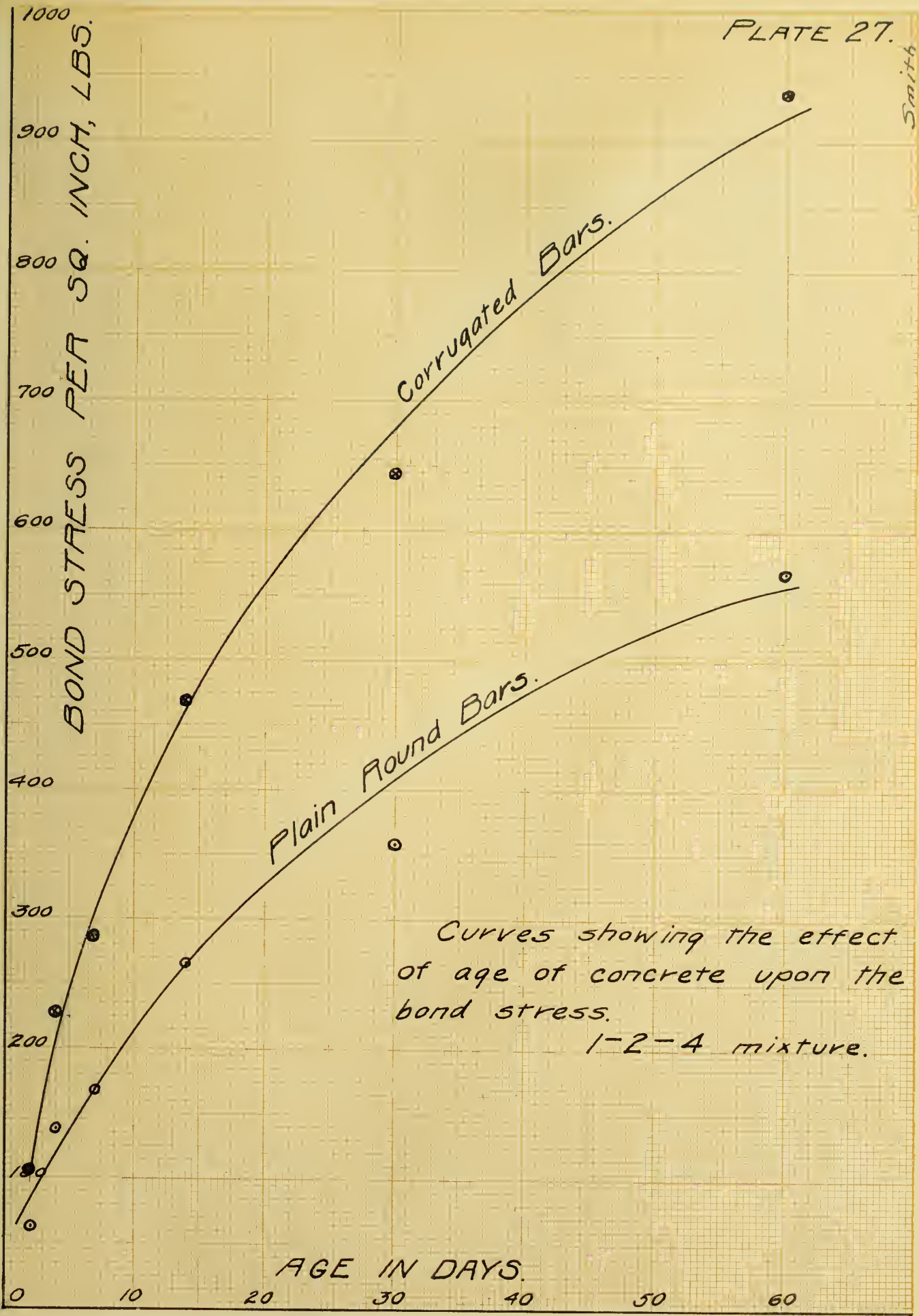
.140



Curves showing the relation between
load and slip of specimens 60 days
old.
1-2-4 mixture.
 $\frac{3}{4}$ inch corrugated
rods.

Slip in inches.

lbs per sq. in.

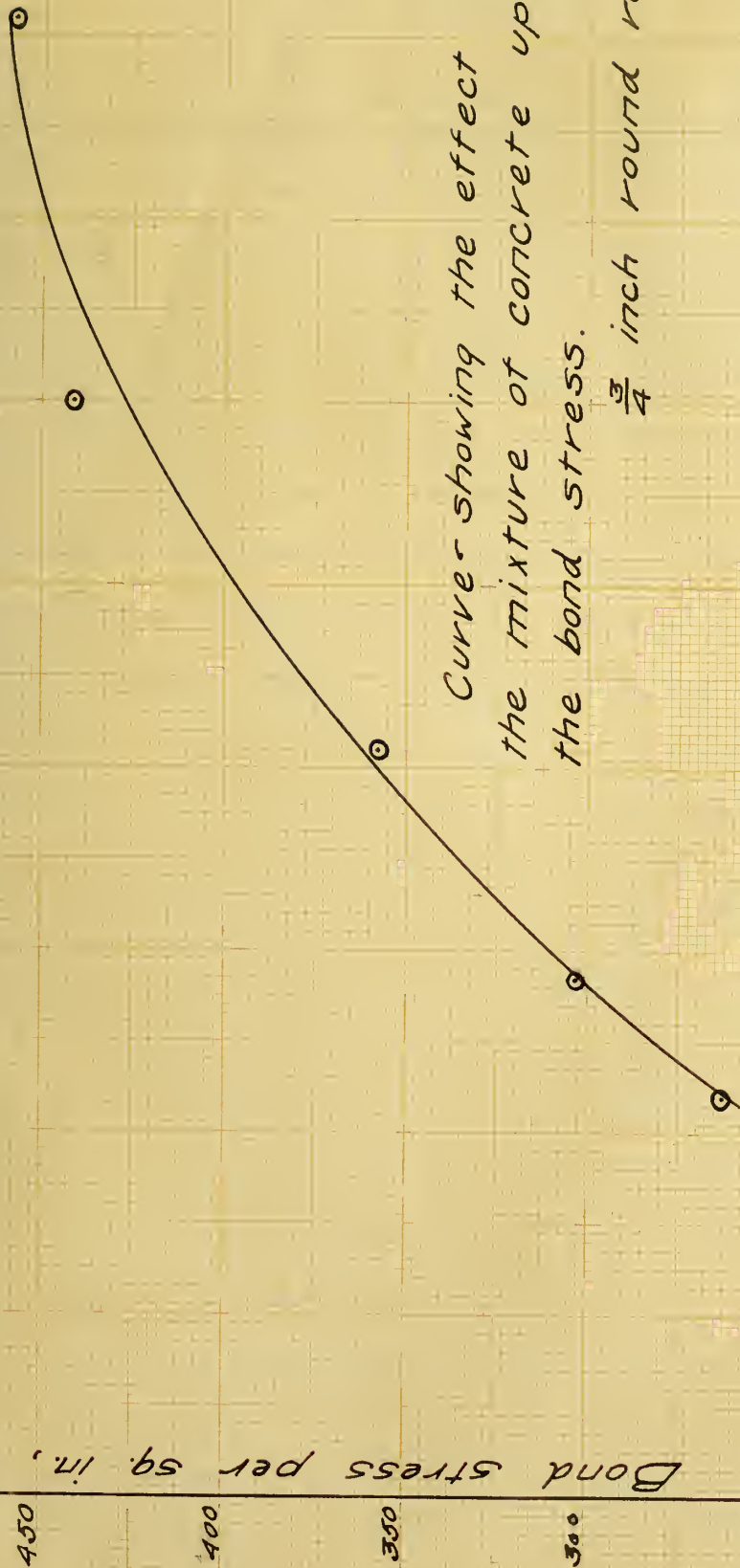


Curves showing the effect of age of concrete upon the bond stress.

1-2-4 mixture.

Bond stress per sq. in., lbs.

Percentage of cement to sand and stone.



Curve showing the effect of the mixture of concrete upon the bond stress.
 $\frac{3}{4}$ inch round rods.

V. CONCLUSIONS.

1. Discussion and Interpretation of Results.

Tables I and VI, and Plates 1 to 5 inclusive, show the effect of the mixture of the concrete upon the bond stress. It is seen by the summary that the bond stress increases with the richness of the mixture, the 1-1-2 mixture giving results 74 per cent. greater than the 1-4-8 mixture.

Tables II and VI and Plates 6 to 9 inclusive, show the effect of storage upon bond stress. These results do not show a very large variation for the different methods of storage, and it can therefore be supposed that the conditions of storage do not effect the bond stress to any large extent, at least for the age of specimens tested. The specimens stored in air give greater values than those store in damp sand, but the difference is not so great that it can be stated that such would always be the case.

Tables III and VI and Plates 10 to 14 inclusive, show the results obtained by the use of various sizes of reinforcing bars. The bond stresses for the different sizes do not vary to any large extent, except that for the 3/4-in. bars, and this is probably due to the fact that these specimens were made from a different batch of concrete from the rest. It seems evident from these results that the unit bond stress is not effected by the size of the bar, but by the condition of its surface.

Tables IV and VI and Plates 15 to 21 inclusive, show

results of age of concrete on bond stress for plain rods, and Tables V and VI and Plates 22 to 28 inclusive, show the same relation for corrugated bars. It is seen that the unit bond stress runs considerable higher all the way through for the corrugated bars than for the round ones. As would be expected, the bond stress increases with the strength of the concrete, being quite small for the 2 and 4 day tests, due to the greenness of the concrete.





UNIVERSITY OF ILLINOIS-URBANA



3 0112 086825731